



### BEAM PROPERTIES

BEAM TYPE	WIDTH (in.)		DEPTH (in)	AREA (in <sup>2</sup> )	I (in <sup>4</sup> )	Y <sub>b</sub> (in)	Y <sub>t</sub> (in)	S <sub>b</sub> (in <sup>3</sup> )	S <sub>t</sub> (in <sup>3</sup> )	WEIGHT (lbs/ft)	MAX. SPAN (ft)
	Nom.	Act.									
B36-24	36.0	35.5	24	543	36609	11.82	12.18	3097	3006	566	73
B36-27	36.0	35.5	27	573	50267	13.29	13.71	3782	3666	597	83
B36-30	36.0	35.5	30	603	66503	14.77	15.23	4503	4367	628	91
B36-33	36.0	35.5	33	633	85453	16.24	16.76	5262	5099	659	96
B36-36	36.0	35.5	36	663	107251	17.72	18.28	6053	5867	691	105
B36-39	36.0	35.5	39	748	135514	19.24	19.76	7043	6858	779	115
B36-42	36.0	35.5	42	784	164680	20.73	21.27	7944	7742	817	122
B36-45	36.0	35.5	45	820	197374	22.21	22.79	8887	8661	854	129
B36-48	36.0	35.5	48	856	233759	23.70	24.30	9863	9620	892	135

#### NOTES:

- Above drawing is not to scale.
- See Dwg. No. 4.2.8 for shear key details.
- Maximum Span lengths are approximate and are based on the following assumptions:
  - $f'_c = 6500$  psi (Precast)
  - $f'_{ci} = 4500$  psi (Precast)
  - $f'_c = 4000$  psi (5" thick Cast-in-Place Composite Deck)
  - Final Allowable Tension at bottom of beam is equal to  $0.0948\sqrt{f'_c}$  ksi.
  - HL-93 Live Load
  - Time-Dependent Losses of Article 5.9.5.3 of the AASHTO-LRFD were used.
  - The CP-PL2 Barrier was assumed on the bridge.
  - 3.5" thick HMA wearing surface.
  - 0.6" diameter low relaxation strands.
  - The factor "k" in the Live Load distribution factor equation was taken as 1.5
- Weights of beams do not include the weight of the solid sections located at the transverse ties. Include the weight of the solid sections for design.
- Thickness of top flange may have to be increased in order to develop sidewalk/safety curb barrier reinforcement (see Dwg. No. 4.3.2). The Designer will have to calculate and use the modified beam properties in such cases.



LRFD BRIDGE  
MANUAL, PART II

## STANDARD 36" WIDE BEAMS

PRECAST CONCRETE BOX BEAMS

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