NOTES:

Maximum span lengths are approximate and based on the following assumptions:

- $f'c = 8000$ psi (Precast)
- $f'ci = 6000$ psi (Precast)
- $f'c = 4000$ psi (8” cast-in-place composite deck)
- Final allowable tension at bottom of beam is equal to 0.0948 $f'c$ ksi.
- HL–93 Live Load
- Time–Dependent Losses of Article 5.9.5.3 of the AASHTO–LRFD were used.
- The S3–TL4 railing was assumed on the bridge with safety curbs.
- 3” thick HMA wearing surface.
- Four beam cross section with two design lanes was assumed for design of 96”–114” wide beams.
- Three beam cross section with two design lanes was assumed for design of 120”–144” wide beams.
- Interior beam with no skew.
- No utility loads.
- Load Distribution was calculated based on Section 3.5.4 of Part I of the Bridge Manual.
- 0.6” diameter low relaxation strands.